Fracture Critical Bridge Inspection Report

NBI Bridge No.: 03024

Local ID: O-127

Route DOUGLAS BLVD. over WEST ELM CREEK City of Oklahoma City



Prepared for:

Oklahoma Department of Transportation Field Division 04

Inspection Date:

1/9/2017

Report Prepared By: **BURGESS & NIPLE, INC.**

5085 Reed Rd. Columbus, Ohio 43220 614-459-2050



BURGESS & NIPLE

Engineers ■ Surveyors ■ Planners

BURGESS & NIPLE

5085 Reed Road | Columbus, OH 43220 | 614.459.2050

Mr. Wes Kellogg, P.E. Field Service Engineer

Oklahoma Department of

Transportation

200 Northeast 21st Street

Oklahoma City, OK 73102-3204

Re: Fracture Critical Bridge Inspection Report

Structure No.: 14N3160E1170001

Local ID: O-127 NBI No.: 03024

S. Douglas Blvd. over West Elm Creek

Cleveland County, District 2

February 8, 2017

Dear Mr. Kellogg:

Burgess & Niple (B&N) performed a fracture critical and routine inspection of the above referenced bridge on January 9, 2017. The bridge is a one span structure (photos 1 and 2) with panel points numbered south to north and consisting of:

Span 1: 61-foot riveted pony truss

The limits of the inspection were from the south abutment to the north abutment. Inspection team members included Michael Seal, PE (Team Leader), and Mike Kronander, EI.

The bridge is currently open with a 7-ton load restriction as per the 2013 load rating analysis (photos 3 and 4).

This report includes appendices containing:

- Condition photographs
- Oklahoma DOT Bridge Inspection Form
- PONTIS element report
- FCM Inspection Frequency worksheet

The current and previous NBI ratings for the bridge are:

NBI Item	Previous Rating (2016)	Current Rating (2017)		
NBI Item 58 (Deck)	6 = Satisfactory	6 = Satisfactory		
NBI Item 59 (Superstructure)	5 = Fair	5 = Fair		
NBI Item 60 (Substructure)	4 = Poor	4 = Poor		
NBI Item 61 (Channel)	4 = Protection Undermined	4 = Protection Undermined		
Sufficiency Rating	20.3 (SD,FO)	20.3 (SD, FO)		

The bridge is structurally deficient and functionally obsolete.



RECOMMENDED ACTIONS, in order of decreasing priority, are as follows:

Priority Code **CX** – Bridge condition is bad enough that there is a possibility of failure of a major structural component if repairs are not completed within the next few days.

• There are no CX repair items required at this time.

Priority Code **PX** – Bridge condition is such that immediate repair is not necessary, but should be completed within the next several weeks or months.

- Stand up the northeast bridge end marker laying on the ground.
- Abrasively clean and paint areas of corrosion and significant section loss, predominantly on the bottoms of stringers at floor beam connections and both abutments.
- Apply mortar to fill in voids for both masonry abutment walls.
- Install countermeasures along the base of the north abutment retaining wall.
- Repair patches and add asphalt to any potholes in the north approach roadway 100 feet from the bridge.

Priority Code **FX** – Bridge condition is such that repair should not be necessary any time soon, monitor during future inspections.

- Monitor any further distortion or defects on the U3, west truss gusset plate.
- Monitor any further section loss of the steel piles at the north abutment.
- Monitor scour conditions of the bridge upstream and downstream along the channel embankments.

In addition to these recommendations it is recommended that this structure be inspected on a 24 month Routine/Fracture Critical inspection frequency and a 24 month Other/Special inspection frequency.

We thank you for the opportunity to provide our engineering services. Please contact me if you have any questions or comments.

Sincerely,

BURGESS & NIPLE, INC.

Michael A. Seal, PE

Team Leader

Attachments



SIGNIFICANT FINDINGS are as follows:

NBI Item 36 – Traffic Safety (5 = Fair condition)

- PX The northeast bridge marker sign is currently laying on the ground (photo 4).
- A "One Lane Bridge Ahead" sign exists at the north approach (photo 5).
- W-beam guardrail is in place across the full length of the bridge being placed in front of the old steel bridge railing. The ends of the guardrail are turned down and buried into the ground at the north end of the bridge.
- All of the traffic safety items meet current standards for a non-National Highway System roadway.

NBI Item 58 – Deck (6 = Satisfactory condition)

Driving Surface – (6 = Satisfactory condition)

- Moderate wear and longitudinal checks and splits occur throughout the timber runners of the deck. The defects appear heavier towards the ends of the bridge (photo 6). The edge runners appear warped and bowed with cracking in random locations.
- The transverse planks have moderate checks along their entire length.

Soffit – (6 = Satisfactory condition)

• No significant defects were observed in the underside of the deck.

NBI Item 59 – Superstructure (5 = Fair condition)
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Fracture Critical Member Rating Summary					
Floor Beam	6 = Satisfactory condition				
Truss Lower Chord	6 = Satisfactory condition				
Truss Web Members	6 = Satisfactory condition				

Stringers – (5 = Fair condition)

- **PX** Stringer 5 has up to 1/4-inch section loss to the bottom face of the bottom flange at the connection to the top flange of the north abutment steel pile cap (**photo 7**). Stringers 3, 4 and 6 only have up to 1/16-inch loss to their bottom flanges. Section loss to stringers at intermediate floor beam connections is typical throughout the bridge.
- Surface corrosion is typical throughout the entire length of the stringers.

[FCM] Floor Beams – (6 = Satisfactory condition)

• Typically, the bolt shanks are not long enough to extend all the way through the bolt nuts at floor beam/truss connections (photo 8). At least half of the bolts at connections exhibit this condition. A mixture of square (bearing type bolts) and hex (friction bolts) headed bolts exist at the floor beam connections.

Floor beams typically have surface corrosion throughout their flanges and webs.

Floor System Bracing – (6 = Satisfactory condition)

- Lower lateral bracing rods are bent at the following locations:
 - Between floor beams 3 and 5, under stringers 3 and 4 (photo 9).
 - Between floor beam 6 and the north abutment under stringer 6.

Truss Upper Chord – (5 = Fair condition)

- **FX Member Alignment** The gusset plate at U3, west truss, has a local bow of up to 1 1/8 inches from previous impact damage. This condition has not changed from the last inspection.
- Minor pitting up to 1/32-inch deep is present to all surfaces of the truss upper chord, with surface corrosion throughout.

[FCM] Truss Lower Chord – (6 = Satisfactory condition)

- Minor pitting and surface rust with no significant loss occurs throughout the truss lower chord.
- LOL2 of the west truss was replaced with the batten plates being welded to the lower chord bottom angle leg (photos 10). This condition creates a category E' fatigue prone detail.

[FCM] Truss Web Members – (6 = Satisfactory condition)

- Minor pitting with surface rust occurs throughout truss web members.
- Bridge railing is welded to the truss web members with no defects or cracks observed at the time of inspection.
- Truss members M1L2, U2L2, and L2U3 have all been replaced for the west truss due to previous impact damage.

Truss End Posts – (6 = Satisfactory condition)

- A welded plate repair occurs for end post LOU2 at LO for the west truss.
- Minor pitting with surface rust occurs throughout truss end posts.
- Bridge railing is welded to all end posts with no defects or cracks observed at the time of inspection.

Paint/Coating System – (0 = Failed condition)

• **PX** – The paint system has failed throughout the truss and steel floor system of the bridge with surface corrosion common (photo 11).

Load Deflection - (7 = Good condition)

No visual defects were observed during live loads traveling over the bridge.

NBI Item 60 – Substructure (4 = Poor condition)

Abutments - (5 = Fair condition)

• **PX** – Both masonry abutment breastwalls exhibit open joints with no mortar where soil spills through the openings (**photo 12**). This has caused some undermining and settlement under the north approach pavement due to the loss of fill.

• **FX** – Located approximately 1-foot above the ground line, pile 2 at the south abutment in the southwest corner of one of the flanges has up to 1/8-inch deep section loss to the overall thickness of the flange **(photo 13)**. Pile 3 at the same abutment has up to 1/16-inch loss.

Bearings – (6 = Satisfactory condition)

• Surface corrosion and light amounts of pitting exist on all truss bearings. The truss rests on steel bearing plates at each end of both steel bent caps (photo 14).

NBI Item 61 – Channel and Channel Protection (4 = Protection Undermined condition) Channel Scour – (4 = Poor condition)

- **PX** An area of 1 1/2 feet transversely and 7 feet longitudinally on the northwest corner of the old concrete abutment in front of the existing north abutment has an exposed rock foundation due to channel scour **(photo 15)**. Concrete wear from channel scour has effected all along the front face of the base of the north abutment breastwall up to 2 feet above the waterline. The north abutment is supported on steel piles and most likely does not rely on the old abutment; however, failure of the old abutment would cause a restriction in the channel.
- **FX** The upstream and downstream channel embankment exhibit significant sloughing up to 15 feet deep (**photo 16**).
- Rock channel protection was previously installed in front of the north abutment embankment to help stabilize the slope. It appeared to be functioning as intended.
- The northeast wingwall has previous channel scour around the end of the wall. Riprap has been installed as a countermeasure, and this condition has not changed since the previous inspection.

Embankment Erosion – (5 = Fair condition)

• **PX** – The northeast corner of the north approach roadway is undermined for up to 2 feet of penetration due to erosion from soil spilling through the open joints of the masonry breastwalls (**photo 17**).

Debris – (6 = Satisfactory condition)

• A debris island occurs downstream of the bridge but has not yet significantly affected the flow of channel going under the bridge.

Vegetation – (7 = Good condition)

 Channel banks are well vegetated with scour undercutting some of the trees along the stream.

NBI Item 72 – Approach (5 = Fair condition)

Approach Alignment – (6 = Satisfactory condition)

• A slight horizontal curve exists at the north approach roadway, while the south approach roadway has a much sharper turn at the bridge.

Approach Roadway Condition – (5 = Fair condition)

• **PX** – Extensive patches and potholes exist on the south approach roadway approximately 100 feet north of the bridge **(photo 18)**.

Approach Roadway Settlement – (5 = Fair condition)

• **PX** – The northeast corner of the north approach roadway is undermined for up to 2 feet of penetration due to erosion from soil spilling through the open voids of the masonry abutment (photo 17).

NBI Item 113 – Scour Rating (4 = Foundations Stable for Calculated Scour) No change to scour rating is recommended.

- **PX** –The northwest corner of the old north concrete abutment has an exposed the rock foundation due to scour **(photo 15)**.
- Previous channel scour has caused erosion of the channel embankments upstream and downstream of the bridge (photo 16).
- Minor erosion occurs behind the northeast wingwall with added riprap in place as a countermeasure.

Structure #

NBI#



Photograph 1 - End view looking north.

NBI#	Structure #	County	Fac. Carried	Fac. Intersected	Insp. Date
03024	14N3160E1170001		DOUGLAS BLVD.	WEST ELM CREEK	1/9/2017



Photograph 2 - Elevation view looking southwest.

NBI#	Structure #	County	Fac. Carried	Fac. Intersected	Insp. Date	
03024	14N3160E1170001		DOUGLAS BLVD.	WEST ELM CREEK	1/9/2017	



Photograph 3 - Looking north at 7-ton load posting sign at south approach.

NBI#	Structure #	County	Fac. Carried	Fac. Intersected	Insp. Date
03024	14N3160E1170001		DOUGLAS BLVD.	WEST ELM CREEK	1/9/2017



Photograph 4 - Looking south at 7-ton load posting sign at north approach. Note: The northeast bridge marker is laying on the ground.

Structure #

County

NBI#



Photograph 5 - Looking south at "One Lane Bridge Ahead" sign located at north approach.

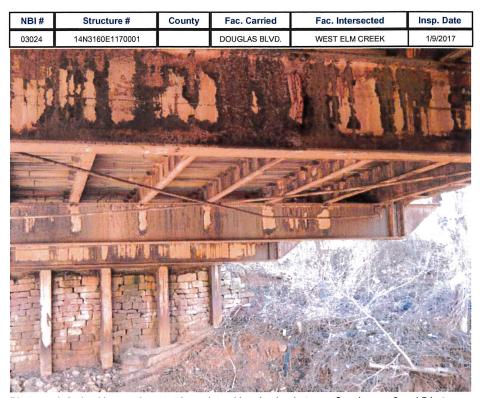


Photograph 6 - Looking north at the south end of the bridge deck. Note: The longitudinal runners have checks and splits along their length, which appear heavier towards the ends of the bridge.

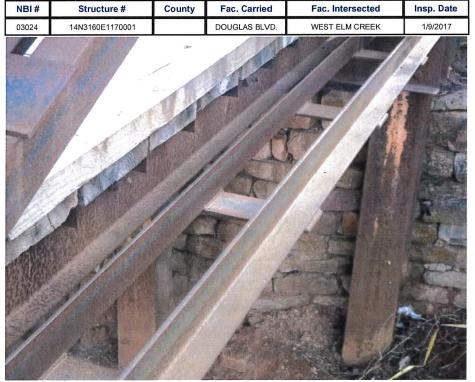
Photograph 7 - Looking north at the stringer 5 bottom flange connection to the north abutment pile cap. Note: Up to 1/4-inch loss to bottom flange.



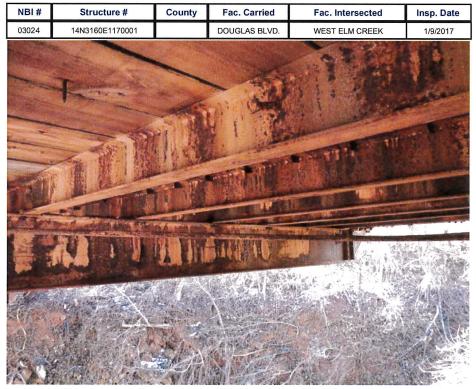
Photograph 8 - Looking south at east truss L2, bolted connection of vertical to floor beam. Note: Bolt shanks are not long enough to extend through nut. Typical to all floor beam connections.



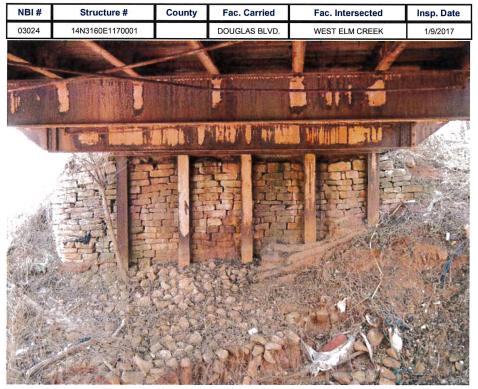
Photograph 9 - Looking southwest at lower lateral bracing bar between floor beams 3 and 5 between stringers 3 and 4. Note: Bar is bent down, no change from last inspection.



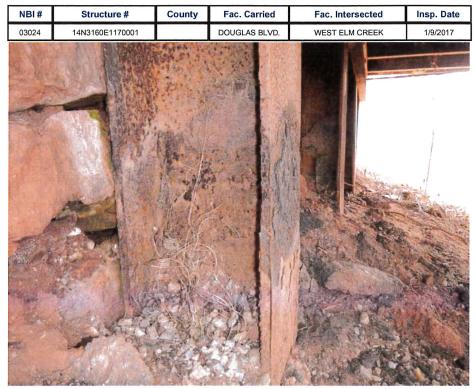
Photograph 10 - Looking southeast at west truss, lower chord. Note: L0L2 has been replaced with batten plates welded to lower chord in this section.



Photograph 11 - Looking southwest at overall typical condition of steel floor system with corrosion and failing paint throughout.



Photograph 12 - Looking south at overall view of north abutment breastwall. Note: Masonry blocks have no mortar between stones throughout abutment wall.



Photograph 13 - Looking northeast at southwest corner of pile 2 at north abutment. Note: Section loss up to 1/8-inch deep on south flange for approximately a 4-inch height of the steel bent.



Photograph 14 - Looking northeast at L0, west truss. Note: The steel plate bearings exhibit surface corrosion with light amounts of minor pitting.



Photograph 15 - Looking southeast at base of the old concrete abutment in front of the current north abutment. Note: east end of old abutment has rock foundation exposed.



Photograph 16 - Looking east downstream of bridge. Note: channel slopes are sloughing heavily and washing out due to channel scour.



Photograph 17 - Looking west at the north approach roadway which is undermined up to 2 feet at the northeast corner of the bridge up due to erosion.



Photograph 18 - Looking northwest approximately 100 feet north of the bridge. Note: Extensive patches and potholes exist in the south approach roadway.

NBI No.: 03024 Structure No.: 14N		ocal ID:O-127	Suff. R	ating: 20.3	3	Health Index:
IDENTIFICATION	5100E1170001 L	Jean 1D.0-127			OM .	33.1
Description: IDENTIFICATION 61' PONY TRUSS SPAN		Type Insp Req.	Insp Done	INSPECTION Freq:	Insp. Date:	Next Insp.:
1. State:Oklahoma 2. SHD District:	Division 3	NBI:	Y	24	1/9/2017	-
3. County Code: CLEVELAND 4. Place Code: O		FC Freq.: Y	Y	24	1/9/2017 1/9/2017	1/9/2019 1/9/2019
Admin. Area: Cnty. District 2		UW Freq.: N	N	NA	NA	NA
5. Inventory Route (Route On Structure): 1 - 5 - 1 -	N3160 - 0	OS Freq.: Y	N	24	1/4/2016	1/9/2018
6. Feature Intersected: CREEK						1/2/2010
9. Location: .1S OF 149TH ON DOUGLAS 11. 13. LRS Inv. Route./ Subroute.: -1 -1 16. Latitude: 35 19 06.08 17.	AS BLVD. (1408C) Mile Post: 0.100 mi Longitude: 097 22 14.23 Border Br. #: Unknown TERIALS	12. Base Hwy Network 21. Custodian: 04City// 26. Functional Class: 100. Defense Highway 102. Dir. of Traffic:3 1- 104. Highway System: 110. National Truck Ne	S: Not on Base N Municipal Hwy A 09 Rural Local ; 0 Not a STRAH -lane Br for 2-wa; 0 Not on NHS	37. H INET h 101. y 103. 105.	oll Facility: 3 On f owner: 04City/Mun listorical Sig.: 2 Br Parallel Structure: 1 Temp. Structure: No Fed. Land Hwy 0 N	icipal Hwy Agenc eligible for NRHP No bridge exists of Applicable (P) N/A (NBI)
Steel Truss-Thru		Tro. Ivational Track Ive	etwork. J Not par	it Of Ha: 112.	TVDIS Ecilgiii. Ecilg	, Enough
44. Approach Span Material and Design Type Not Applicable (P) Not Applic 45. No. of Spans Main Unit: 1 46. No. of Approach 107. Deck Type: 8 Wood or Timber 108A. Wearing Surface: 7 Wood or Timber 108B. Membrane: 0 None		58. Deck: 6 Satisfacto 62. Culvert: N N/A (I Flowline Notes: 1/9/17 - FL to top of Ti	NBI) 61. 0		60. Sonel Protection: 4 Pr	ub.: 4 Poor otection Undermined
108C. Deck Protection: None		FL = 25.3' to top of dec				
AGE AND SERVICE	1		LOAD	RATING ANI	POSTING	
	Reconstructed: Unknown	31. Design Load: 0 Un			Posting status: P Po	sted for load
28A. Lanes on: 1 28B. Lanes Under: 0	19. Detour Length: 4.0	1				
29. ADT: 300 30. Year of ADT: 2014	109. Truck ADT %: 10	64. Operating Rating ((H / HS / 3-3):	12.3	22.2	-1.1
42A. Type of Service on: 1 Highway		66. Inventory Rating ((H/HS/3-3):	7.4	13.3	-1.1
42B. Type of Service under: 5 Waterway		65. Inv. Rating Metho				
OPOLETRIC E : T		70. Posting: 0 >39.9%	below	Date	Rated: 4/18/2013	3
GEOMETRIC DATA	<u>1</u>		PROPO	OSED IMPRO	VEMENTS	
10. Inv. Rte. Min. Vert. Clr.: 328.1 ft 32. Approach Roadway Width (W/ Shoulders): 25.0 ft		94. Bridge Cost: 5	\$245,000	75.	Type of Work: 31	Repl-Load Capacit
	n: 0 No median	95. Roadway Cost: 5			Lgth. of Improvm	
B 3 0300	Flared: 0 No flare	PARKETER COMMUNICATION AND PROPERTY AND PROP	\$390,000		Future ADT: 480	
47. Inv. Rte. Total Horiz. Clr.: 15.7 ft	riarea. 6 146 fiare	97. Year of Cost Est.:	: 2009	113	. Year of Future AD	01: 2034
48. Length Maximum Span: 60.0 ft 49. Structu	re Length: 61.0 ft		<u>N</u>	NAVIGATION	DATA	
The state of the s	idewalk Width R: 0.0 ft	38. Navigation Cont			**	0.00
51. Width Curb to Curb: 15.7 ft 52. Width	Out to Out: 17.7 ft	39. Vertical Clearand 111. Pier Protection:			Horizontal Clearan Lift Bridge Vert. C	
53. Minimum Vertical Clearance Over Bridge: 328.1 ft		TIT. TICI TICICCION.	1 Not Required			7.01.0.0 It
54A/54B. Min. Vert. Underclearance: N Feature not hwy	or RR 0.0 ft	264 P-id P-il-06	N. I	APPRAISA	and the same of th	0.0.111
<u>N/E</u> <u>S/W</u>		36A. Bridge Rail: 0 S 36B. Transition: 0 S			Approach Rail: Approach Rail Ends	0 Substandard
<u>Meas.</u> -1 -1 -1 -1	-1 -1	67. Str. Evaluation:				ntolerable - Correct
Post. DO NOT U DO NOT U DO NOT U DO N	NOT UDO NOT U-1	69. Underclearance,				molerable Correct
55A/55B. Minimum Lateral Undrclearance R: N Feature	not hwy or RR 0.0 ft	71. Waterway Adequ				
56. Minimum Lateral Undrclearance L: 0.0 ft	,	72. Approach Aligni	353			
		113. Scour Critical:	4 Stable, needs a	ction		
200c. Temperature: 60	214a. Posted Weight Lim	nit: 070707		1 243. Gird	ler Spacing/Number	: -1.0 / -1
200d. Weather: PARTLY CLOUDY	b. Posted Speed Limit				n Lengths:	-1.0 · -1
201. Structural Steel ASTM Desig.: -1 -1	c. Narrow/One Lane I			61	-1	-1
202. Waterproof Membrane :-1	d. Vertical Clearance			-1	-1	-1
Date Installed: 1/1/1901	Advanced Warning	; Sign: NO		-1	-1	
203. Type Exp. Dev. : _					ler Depth: -1.000 e of Overlay: Ti	mber Running Plank
lan. =	e. Navigation Lights :	NO			rlay Thickness: 2.0	
204. Type of Handrail: Metal Railing (other)	Working/Not Work			~		17/2013
205. Material and Quantity: -1.0 208. Type of Abutment: Skeleton	215. Overpass : O - ACO				rlay Depth Changed	
Type of Foundation: Steel Piling	221. Substructure Cond.				ective Systems : 1:	_
209. Type of Pier / Found.:	222. Fill over RCB:	-1		2: _	3:	
	223. Appr. Slab/Rdwy Co	ond.: Satisfactory		4: _	5:	
210. Foundation Elev1.0 -1.0					of Field Splices w/	
-1.0 -1.0 -1.0	225. Paint Type :	Red Lead Ready			ar Crit. POA exists?	
	Overcoat:	Not Applicable		250. Culv	vert Headwall Dist.:	-1.0
211. Wear. Surf. Prot. System : _ Date Installed : 1/1/1901	226. Date Painted: 227. Paint Coloring:	3001 Silver		256. Cha	n. Profile Up/Down	Stream?:
213. Utilities Attached : Communication	233. Deck Forming:	Bilvei			iePROS Auto. Trucl	_
-1 -1 -1				258. Plan	s w/ found. are in fi	le at ODOT:
-1 -1 -1	238. School Bus Rte: De 240. Appr. Roadway Type			263. Inter	r Eval. is in file at 0 rchange at Intersecti rstate Milepoint:	

OKLAHOMA DEPARTMENT OF TRANSPORTATION -

-1.00

264. Interstate Milepoint:

Bridge Inspection Report

OKLAHOMA DEPARTMENT OF TRANSPORTATION -

Bridge Inspection Report

Suff. Rating: 20.3

Health Index: 35.7

NBI No.: 03024

Structure No.:14N3160E1170001

Local ID:O-127

SD

Inspection Date: 1/9/2017 Reported By: MSEAL Invoice No.: Inspected With: Agency:

Michael Seal Digitally signed by Michael Seal DN: cn=Michael Seal, o=Burgess and Niple, ou=Facility Inspection Group, email=michael.seal@burgessniple.com, c=US Date: 2017.03.13 12:56:43 -04'00'

Structure / Inspection Notes

61' long riveted pony truss.
OS inspection items include: Undermining of edge of approach roadway at north abutment; Upstream channel eroding north approach embankment; Welds on truss members; Section loss to steel piles at abutments; Section loss to stringers at abutments on bottom flanges.

PX - Stand up the northeast bridge end marker laying on ground; Abrasively clean and paint areas of section loss and corrossion; Apply mortar in masonry abutment voids; Install countermeasures along base of old north abutment; Repair patches and potholes in north approach roadway 100 feet from bridge.

FX - Monitor: any further distortion or defects of U3, west truss gusset plate; section loss on steel piles at north abutment; scour conditions upstream and downstream of bridge.

Elm.	Env	. Description	Un.	Qty.	Qty.St. 1	% 1	Qty.St. 2	% 2	Qty.St. 3	% 3	Qty.St. 4	% 4	Qty.St. 5	% 5
31	4	Timber Deck	(SF)	958	335	35 %	479	50 %	144	15 %	0	0 %		0 %
113	1	Steel Stringer/Floorbeam	(LF)	488	0	0 %	0	90 %	488	10 %	0	0 %	0	0 %
120	1	Steel Truss (Pony)	(LF)	122	0	0 %	110	90 %	12	10 %	0	0 %	0	0 %
162	1	Steel Gusset Plate	(EA)	22	0	0 %	22	100 %	0	0 %	0	0 %	0	0 %
202	1	Steel Column or Pile Extension	(EA)	10	0	0 %	8	80 %	2	20 %	0	0 %	0	0 %
217	4	Masonry Abutment	(LF)	40	0	0 %	0	0 %	40	100 %	0	0 %	0	0 %
231	4	Steel Pier Cap	(LF)	36	0	0 %	36	100 %	0	0 %	0	0 %	0	0 %
313	1	Fixed Bearing	(EA)	4	4	100 %	0	0 %	0	0 %	0	0 %	0	0 %
330	1	Metal Bridge Railing	(LF)	244	122	50 %	0	0 %	122	50 %	0	0 %	0	0 %
510	1	Wearing Surfaces	(SF)	610	0	0 %	585	96 %	25	4 %	0	0 %	0	0 %
515	1	Steel (Superstructure) Protective Coating	(SF)	5,155	0	0 %	0	0 %	0	0 %	5,155	100 %	0	0 %
918	1	Steel (Substructure) Protective Coating	(SF)	454	0	0 %	0	0 %	0	0 %	454	100 %	0	0 %
919	1	Steel (Railing) Protective Coating	(SF)	716	716	100 %	0	0 %	0	0 %	0	0 %	0	0 %
957	4	Pack Rust	(EA)	1	0	0 %	1	100 %	0	0 %	0	0 %	0	0 %
962	1	Superstructure Traffic Impact	(EA)	1	0	0 %	1	100 %	0	0 %	0	0 %	0	0 %
963	4	Steel Section Loss	(EA)	1	0	0 %	1	100 %	0	0 %	0	0 %	0	0 %
965	1	Debris	(EA)	1	0	0 %	1	100 %	0	0 %	0	0 %	0	0 %
968	1	Erosion	(EA)	1	0	0 %	1	100 %	0	0 %	0	0 %	0	0 %
970	1	Wing	(EA)	2	0	0 %	1	50 %	1	50 %	0	0 %	0	0 %

Additional Elements

Elem.	Element Notes (Include Size and Location of Deterioration
31	The transverse planks have moderate checks along their entire length.
113	PX-Stringer 5 at north abutment has up to 1/4" section loss to the bottom flange. Stringers 3, 4 and 6 at north abutment have up to 1/16" loss to their bottom flanges. Section loss to stringers at intermediate floor beam connections is typical throughout the bridge. Surface corrosion is typical throughout the entire length of the stringers.
120	W L0L2, W M1L2, W U2L2 and W U3L2 replaced with welded connections and batten plates. Bridge railing welded to truss web members. Areas of pitting with failing paint and surface corrosion throughout.
162	FX – The gusset plate at U3, west truss, has a local bow of up to 1 1/8 inches from previous impact damage. This condition has not changed from the last inspection.
	FX – Located approximately 1-foot above the ground line, pile 2 at the south abutment in the southwest corner of one of the flanges has up to 1/8-inch deep section loss to the overall thickness of the flange. Pile 3 at the same abutment has up to 1/16-inch loss.
	PX – Both masonry abutment breastwalls exhibit open voids with no mortar where soil spills through the openings. This has caused some undermining and settlement under the north approach slab due to the loss of fill.
231	Surface corrosion throughout with minor pitting.
313	Surface corrosion and light amounts of pitting exist on all truss bearings. The truss rests on steel bearing plates at each end of both steel bent caps
	W-beam guardrail is in place across the full length of the bridge being placed in front of the old steel bridge railing. The ends of the guardrail are turned down and buried into the ground at the south end of the bridge. Old railing has many areas of failed paint and surface corrosion throughout.
	Moderate wear and longitudinal checks and splits occur throughout the timber runners of the deck. The defects appear heavier towards the ends of the bridge. The edge runners appear warped and bowed with cracking in random locations.
515	PX – The paint system has failed throughout the truss and steel floor system of the bridge with rust and surface corrosion common.
918	The paint has failed throughout most the pier cap and section loss exists on the piles. Limited effectiveness of paint.
919	Protective coating for w-beam is very effective.
957	Minor pack rust at connections of floor beams/truss web verticals and at stringers/floor beams.
962	FX – The gusset plate at U3, west truss, has a local bow of up to 1 1/8 inches from previous impact damage. This condition has not changed from the last inspection.
963	1/4-inch section loss exists at lower portions of piles. Stringers have minor pitting and section loss at random locations of the bottom flanges from 1/16 to 1/4-inch at the abutments.
965	Debris island exists downstream of bridge.

2/17/2017 Page 2 of 3 OKLAHOMA DEPARTMENT OF TRANSPORTATION -

Bridge Inspection Report
Suff. Rating: 20.3 Health Index: SD 35.7

NBI No.: 03024

Structure No.:14N3160E1170001 Local ID:O-127

Elem.	Element Notes (Include Size and Location of Deterioration
	PX – The northeast corner of the north approach roadway is undermined for up to 2 feet of penetration due to erosion from soil spilling through the open voids of the masonry abutment
	The northeast wingwall has previous channel scour around the end of the wall. Riprap has been installed as a countermeasure, and this condition has not changed since the previous inspection.

 Structure No.
 14N3160E1170001

 NBI No.
 03024

 Facility
 Douglas Blvd.

 Feature Int.
 Creek

Overall Score 57
Inspection Frequency 24mo FC/Routine & 24mo OS

12mo FC/Routine & 12mo OS \leq 45 45 < 24mo FC/Routine & 24mo OS \leq 60 24mo FC/Routine > 60

Screening Phase

Five points are given for each of the eight criteria in the screening phase. For each "positive" quality the bridge/FCM possesses, five points are added to its overall score.

		<u>Points</u>
New/Recently Retrofitted or Rehabilitated	No	5
2. Pin and Hangers	No	5
3. Non-redundant Eye bars	No	5
4. Plug Welds or Discontinuous Backup Bars	No	5
5. Active Fatigue Cracks	No	5
6. Susceptibility to Constraint Induced Fracture	No	5
7. Existing Maintenance Problem or Load Posted	Yes	0
8. NBI rating of FCM	6	5
, in the second		35
Scoring Phase		
_		<u>Points</u>
Fabricated under the AASHTO/AWS FCP	No	0
(refer to plans)		
AASHTO Temperature Zone	2	5
(refer to AASHTO LFRD Bridge Design Specifications 2007 Table 6.6.2-1)		
3. ADTT (single lane)	30	10
Was the data field measured?	No	
(refer to inspection report)		
4. Truck traffic is completely prohibited	No	0
(refer to inspection report)		
Fabricated using High Performance Steel	No	0
(refer to plans)		
6. NBI rating of FCM(s)	6	7
Exposed to deicers or hash environment?	No	
(refer to fracture critical report)		
7. Internal Redundancy	No	0
Was analysis performed?	No	
(refer to plans)		
8. Structural Redundancy	No	0
(refer to plans)		
9. Remaining Fatigue Life (years)	≤25	0
(refer to calculations)		
10. Fatigue Detail Category	E or E'	0
(refer to AASHTO LFRD Bridge Design Specifications 2007 Table 6.6.1.2.3-	-1)	
11. Tack Welds to FCM's or in tension zones	Yes	0
(refer to inspection report)	,	
12. Owner's/ Engineer's Discretion	0	0
		22
5 points can be added or deducted to total assessment score at the	owner's/	
engineer's discretion (enter justification here).		