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| ltem | Document | Section No.  | Section Subject  | City Revision   | J&A Comment   | City Comment  |
|------|----------|--------------|--|---|---|---|
| 1    | DCM      | 2.3          | Runoff Calculation Methods   | No minimum SCS, but Rational allowed to 200 acres.  | If both Rational Method and SCS have minimum acreage of 0<br>acres, it should be noted that either can be used at the<br>Engineer's Discretion, up to 200 acres.  | Minimum for SCS method will be 0. Since the minimum acreage for both methods is 0 acres, it is implied that the engineer can use either method without concurrence or approval by the City.   |
| 2    | DCM      | 3.3.1.C      | Inlet Design Flow and Location                                       | If a street/roadway is designed to drain towards a "T" intersection<br>and intersecting street is flowing at 70% or greater street capacity<br>as described in Section 3.2.2 above, then inlets must be placed<br>before the intersection to intercept all of the upstream street<br>discharge. | "Intercept" 100% of flows at T intersection or XX%? Need more<br>clarification from the city. How about "Intercept upstream<br>discharge so that the combined flows do not exceed the<br>recievings street capacity (for Q25)"  | This sentence will be revised to: "At 'T' intersesction locations,<br>storm sewer systems must be provided to intercept runoff in<br>the streets such that the depth of flow at the downstream curb<br>return of the 'T' intersection does not exceed 70% of the street<br>capacity as described in Section 3.2.2 above." The remainder<br>of the paragraph will not be changed.  |
| 3    | DCM      | 3.3.2 Extra  | Curb Inlets (Design 2 Inlets) - Design<br>and Construction Standards | FHWA Inlet capacity methodology will be acceptable provided<br>that the calculated inlet capacities do not exceed those listed in<br>Table 3-1.   | Provide calculations for lower capacities? Wouldn't engineers<br>just want to use those listed in Table 3-1 instead of providing<br>additional calculations?  | The addition of the FHWA methodology was added in<br>response to a question received during the public meetings in<br>2021. We are not asking for additional calculations. The<br>FHWA method calculates inlet interception based on the<br>longitudinal and cross slopes of the street. If FHWA is used<br>for storm sewer inlet design, then the OKC Standard inlet sizes<br>listed in Table 3-1 will be used for the design. |
| 6    | DCM      | 4.3          | Bridge, Culverts, and Other Special<br>Structures - Design Standards | Culvert slopes less than 0.50% must be approved by the City Engineer.   | City Engineer approval should only be needed for pipes<br>smaller than a specified size. There are several cases where<br>RCBs can't be designed at 0.5%. Sizing should be based on<br>flows/velocities.  | This section states that a minimum culvert slope of 0.50% must be used UNLESS the site conditions or slope of the natural channel requires the use of a flatter slope. If that is the case, culvert slopes less than 0.50% must be approved by the City Engineer and have minimum 2.5 fps velocity.   |
| 7    | DCM      | 4.4.1        | Erosion Control and Scour - Maximum<br>Shear Stress                  | A detailed designed of channel protection based on "Design of<br>Roadside Channels with Flexible Linings" (FHWA) or "Hydraulic<br>Design of Energy Dissipators fo Culverts and Channels"<br>(USDOT) shall be provided.  | Need more clarification on these "detailed studies". What is<br>Table 4-1 referenced from?  | The engineer will be required to provide calculations to show maximum shear stress isn't exceeded based on Tables 4-1 and 4-2.  |
| 8    | DCM      | 6.4          | Design and Construction of Detention<br>Ponds - Outlet Configuration | Grass or sod overflow spillways will not be allowed.  | Need to include in this section about compounded outfalls.<br>Barry has commented previously that the maximum allowable<br>pipe size outfall is 18", but that introduces higher chances of<br>clogging and slopes are limited. This should be based on<br>flows/velocities, or up-stream drainage area. | There is a sentence above this paragraph that states outlet control structures can be compound outfalls weirs, culverts standpipes, or any combination therof   |
| 9    | DCM      | 6.5          | Underground Detention  | 4. Outflow must be able to drain directly into an existing storm facility without mechanical assistance.  | This seems shortsighted for downtown development.   | When the City started accepting underground detention almost 10 years ago this was a requirement  |
| 10   | Code     | 16-5.2       | Responsibility Improvements -<br>Stormwater Improvements             | Definitions of Public vs. Private Drainage based on Public vs.<br>Private Streets. PV subject to cursory review. Public and Private<br>storm sewers must be deisgned to public requirements. All<br>private storm sewer is inspected by Public Works Field Services.                            | City should maintain public sewer under public road even if<br>proposed private storm sewer is connecting to public. Property<br>owner shouldn't have to pay out of pocket for road replacemen<br>if sewer fails.   | , The City does maintain public storm sewer under public<br>streets. The property owner or HOA is required to maintain<br>private storm sewer under private streets.  |
| 11   | Code     | 16.16.1.m.iv | FPAT Requirements  | An exception to Subsection 1.j through 1.l above may be made<br>for oil and gas drilling operationsprovided the following special<br>requirements are met   | Does this include gas facilities that are non-habitable?  | 1.m.ii covers permanent structures or appurtenances   |